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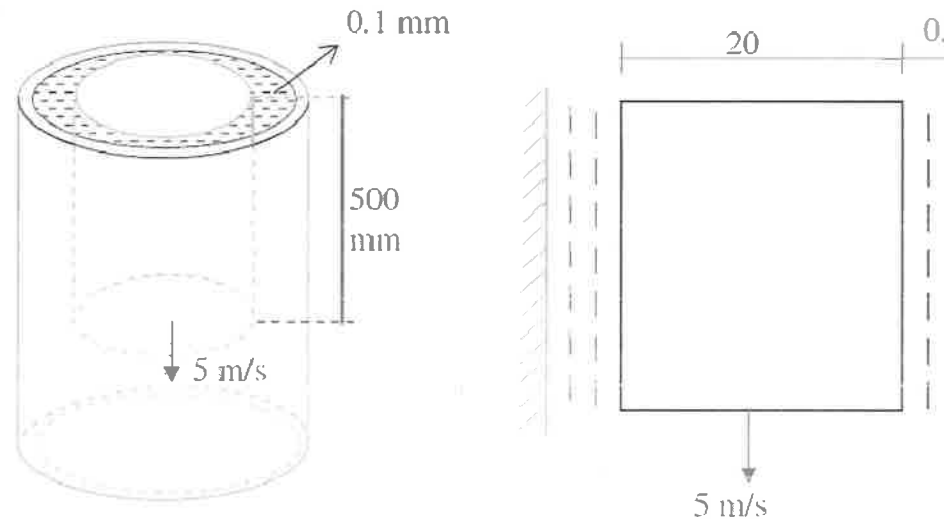
S.E (Production) Sem-III (Rev.) CBGS, 29/11/17

Subject: FLUID MECHANICS AND FLUID POWER

Marks: 80

SYNOPTIC

- Q.6 c A shaft of 20 mm and mass 15 kg slides vertically in a sleeve with a velocity of 5.08 m/s. The gap between the shaft and the sleeve is 0.1 mm and is filled with oil. Calculate the viscosity of oil if the length of the shaft is 500 mm.



$$D = 20\text{mm} = 20 \times 10^{-3}\text{m}$$

$$M = 15\text{ kg}$$

$$W = 15 \times 9.81$$

$$W = 147.15\text{N}$$

$$y = 0.1\text{ mm}$$

$$y = 0.1 \times 10^{-3}\text{mm}$$

$$U = 5\text{m/s}$$

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$$F = W$$

$$F = 147.15 \text{ N}$$

$$\mu = ?$$

$$A = \pi D L$$

$$A = \pi \times 20 \times 10^{-3} \times 0.$$

$$A = 0.031 \text{ m}^2$$

$$\tau = \mu \cdot \frac{U}{Y}$$

$$4746.7 = \mu \times \frac{5}{0.1 \times 10^{-3}}$$

$$\mu = 0.095 \frac{\text{NS}}{\text{m}^2}$$

$$\tau = \frac{F}{A}$$

$$= \frac{147.15}{0.031}$$

$$\tau = 4746.7 \text{ N/m}^2$$

- Q.2 c A large tank of sea water has a door in the side 1 m square. The top of the door is 5 m below the free surface. The door is hinged on the bottom edge. Calculate the total pressure force and centre of pressure.. The density of the sea water is 1033 kg/m³.

Solution: The total hydrostatic force $F = \gamma_{\text{sea water}} A h_c$

$$\gamma_{\text{sea water}} = 1033 \times 9.81 = 10133.73$$

$$\text{Given } A = 1\text{m} \times 1\text{m} = 1\text{m}^2$$

$$h_c = 5 + \frac{1}{2} = 5.5\text{m}$$

$$F = 10133.73 \times 1 \times 5.5 = 55735.5 \text{ N}$$

Acting at centre of pressure ($y_{c,p}$):

From the above $h_c = 5.5\text{m}$, $A = 1\text{m}^2$

$$(I_c)_{xx} = \frac{BD^3}{12} = \frac{1 \times 1^3}{12} = 0.08333\text{m}^4$$

$$h_{c.p.} = h_c + \frac{(I_c)_{xx}}{Ah_c} = 5.5 + \frac{0.08333}{1 \times 5.5} = 5.515\text{m}$$

- Q.3 c A venturimeter has its axis vertical, the inlet & throat diameter being 150 mm & 75 mm respectively. The throat is 225 mm above inlet and $C_d = 0.96$. Petrol of

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specific gravity 0.78 flows up through the meter at a rate of $0.029 \text{ m}^3/\text{sec}$. find the pressure difference between the inlet and throat.

Solution:

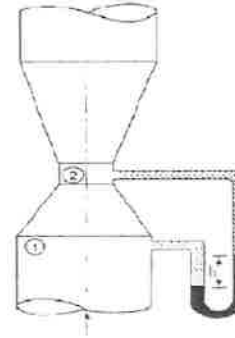


Fig. 1: venturimeter with its axis vertical

The discharge through a venturimeter is given by

$$Q = C_d [a_1 a_2 / \sqrt{a_1^2 - a_2^2}] \sqrt{2gh}$$

Given:

$$C_d = 0.96$$

$$d_1 = 150 \text{ mm} = 0.15 \text{ m}$$

$$d_2 = 75 \text{ mm} = 0.075 \text{ m}$$

$$a_1 = (\pi/4) * 0.15^2 = 0.0177 \text{ m}^2$$

$$a_2 = (\pi/4) * 0.075^2 = 0.0044 \text{ m}^2$$

$$Q = 0.029 \text{ m}^3/\text{sec}$$

By substitution, we have

$$0.029 = 0.96 [0.0177 * 0.0044 / \sqrt{0.0177^2 - 0.0044^2}] \sqrt{2 * 9.81 * h}$$

$$h = 2.254 \text{ m of oil}$$

$$h = (p_1/w + z_1) - (p_2/w + z_2)$$

$$2.254 = [(p_1/w) - (p_2/w)] - [z_2 - z_1]$$

$$2.254 = [(p_1/w) - (p_2/w)] - [0.225]$$

$$p_1/\rho g - p_2/\rho g = 2.479$$

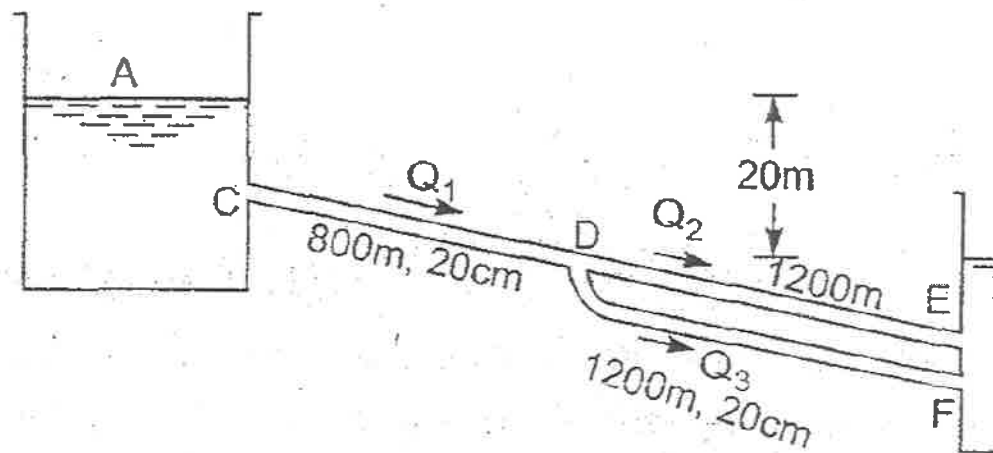
$$\text{Therefore, } p_1 - p_2 = 2.479 * 0.78 * 9810$$

$$= 18969 \text{ N/m}^2 = 18.969 \text{ k N/m}^2 = 18969 \text{ Pa}$$

$$\text{Pr. Difference, } p_1 - p_2 = 18.96 \text{ kPa}$$

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- Q.5 c A pipe of diameter 20 cm and length 2000 m connects two reservoirs, having 08
difference of water levels as 20 m. Determine the discharge through the pipe. If
an additional pipe of diameter 20 cm and length 2000 m is attached to the last
1200 m length of the existing pipe, find the increase in the discharge. Take $f =$
0.015 and Neglect minor losses



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1st case: When a single pipe connects the reservoirs

$$H = 4 f L v^2 / 2gd = 4f L V^2 / 2gd (Q / \pi / 4 d^2)^2$$

$$[\text{as } V = Q / \pi \times d^2 / 4]$$

$$= 32 f L Q^2 / \pi^2 g d^5$$

$$20 = 32 \times 0.015 \times 2000 \times Q^2 / \pi^2 \times 9.81 \times (0.2)^5$$

$$Q = 0.0254 \text{ m}^3/\text{s}$$

2nd case:

Let Q_1 = discharge through pipe CD

Q_2 = discharge through pipe DE

Q_3 = discharge through pipe DF

Length of pipe CD, $L_1 = 800\text{m}$ and its dia. $d_1 = 0.20 \text{ m}$

Length of pipe DE, $L_2 = 800\text{m}$ and its dia. $d_2 = 0.20 \text{ m}$

Length of pipe DF $L_3 = 800\text{m}$ and its dia. $d_3 = 0.20 \text{ m}$

Since the diameters and lengths of the pipes DE and DF are equal. Hence $Q_2 = Q_3$.

Also, for parallel pipes, we have

$$Q_1 = Q_2 + Q_3 = Q_2 + Q_2 = 2Q_2$$

$$[\text{as } Q_2 = Q_3]$$

$$\text{Therefore } Q_2 = Q_1 / 2$$

Applying Bernoulli's equation to points A and B and the flow through CDE, we have

$$20 = 4 \times f \times L_1 \times v_1^2 / d_1 \times 2 \times 9.81 + 4 \times f \times L_2 \times v_2^2 / d_2 \times 2 \times 9.81$$

$$\text{Where } v_1 = Q_1 / \pi \times 0.2^2 / 4 = 4 \times Q_1 / \pi \times 0.04$$

$$v_2 = Q_2 / \pi \times 0.2^2 / 4 = 4 \times Q_2 / \pi \times 0.04 = 4 \times Q_1 / 2 / \pi \times 0.04 = 2 \times Q_1 / \pi \times 0.04$$

$$= 4 \times .015 \times 800 / 0.2 \times 2 \times 9.81 \times (4 \times Q_1 / \pi \times 0.04)^2 + 4 \times .015 \times 1200 / 0.2 \times 2 \times 9.81 \times (2 \times Q_1 / \pi \times 0.04)^2$$

$$= 12394 Q_1^2 + 4647 Q_1^2 = 17041 Q_1^2$$

$$Q_1 = \sqrt{20 / 17041}$$

$$\text{Increase in discharge} = Q_1 - Q = 0.0342 - 0.0254 = 0.0088 \text{ m}^3/\text{s}$$

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- Q.4 c Oil of viscosity 8 Poise and specific gravity 1.2 flows through a horizontal pipe 80 mm in diameter. If the pressure drop in 100 m length of the pipe is 1500 kN/m², determine. 08
1. Rate of flow of oil in lpm.
 2. The maximum velocity
 3. The velocity and shear stress at 10 mm from the wall.

We have $(-\partial p/\partial x)$

$$= 1500 \times 1000 / 100 = 15,000 \text{ N/m}^2/\text{m}$$

$$\text{Average velocity} = u_{av} = (R^2/8\mu)(-\partial p/\partial x) = (0.04^2/8 \times 0.8)(15,000) = 3$$

$$\text{Discharge (q)} = (\pi D^2/4)u_{av} = 0.01885 \text{ m}^3/\text{s}$$

$$= 18.85 \text{ lpm} = 1131 \text{ lpm.} \quad (D=0.08\text{m})$$

$$\text{Max. Velocity (u}_{max}) = 2 u_{av} = 7.0 \text{ m/s (at the center line)}$$

$$\text{Wall shear stress } (\tau_0) = -(\partial p/\partial x)(R/2) = 300 \text{ N/m}^2$$

Total frictional drag for 100m-pipe length (F_D)

$$= \tau_0 \pi DL = 7540 \text{ N} = 7.54 \text{ kN} \quad (L=100\text{m})$$

$$\text{Power required to maintain flow (P)} = F_D \times u_{av} = 28.275 \text{ kW ;}$$

$$\text{Also, } P = q \Delta P = 0.01885 \times 1500 = 28.275 \text{ kW.}$$

Velocity gradient at pipe wall

$$\tau_0 = \mu(\partial u/\partial y)_{y=0}$$

$$(\partial u/\partial y)_{y=0} = (\tau_0/\mu) = 300/0.8 = 3.75/\text{s}$$

$$(10P = 1 \text{ N-m/s}^2)$$

Velocity and shear stress at 10mm from the wall ($y=10\text{mm}$, $r=30\text{mm}$)

$$\text{At } r=30\text{mm, shear stress } (\tau) = \tau_0 (30/40) = 225 \text{ N/m}^2$$

$$\text{OR } (\tau) = (-\partial p/\partial x)(r/2) = 225 \text{ N/m}^2$$

Local velocity (at $r=30\text{mm}$)

$$u = u_{max} [1 - (r/R)^2] = 3.28 \text{ m/s.}$$