

Q.1 F.

$$i = 2 \times 120 = 240 \text{ mm} = 0.24 \text{ cm}$$

$$f = 24 \text{ cm}$$

$$d = 10 \text{ cm}$$

$$f/i = \frac{24}{0.24} = 100$$

$$\text{and } (f+d) = 24+10 = 34 \text{ cm}$$

\therefore Multiplying constant = 100.

Additive constant = 34 cm ≈ 0.34

} Tacheometric constants.

Q.2 a

Looking at the given readings, it is seen that F.B and B.B of CD differ exactly by 180° . Hence, stations C and D are free from local attraction. A and B are the stations affected by local attraction.

Thus, F.B and B.B of CD and F.B of DA are free from error.

$$\text{Correct B.B of DA} = \text{Correct F.B of DA} - 180^\circ$$

$$= 306^\circ 40' - 180^\circ = 126^\circ 40'$$

$$\text{Error in measurement at A} = \text{Observed B.B of DA} - \text{Correct B.B of DA}$$

$$= 126^\circ - 126^\circ 40'$$

$$= -0^\circ 40' 0''$$

$$\text{Correct F.B of AB} = \text{Observed bearing of AB} + \text{Error at A}$$

$$= 74^\circ 20' + 0^\circ 40' = 75^\circ 0' 0''$$

$$\text{Correct B.B of AB} = \text{Correct F.B of AB} + 180^\circ$$

$$= 75^\circ + 180^\circ$$

(Q.2a continued) . . .

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Error in measurement at B

$$= \text{Observed B.B of AB} - \text{Correct B.B of AB}$$

$$= 256^\circ - 255^\circ = +1^\circ$$

Correct F.B of BC

$$= \text{Observed F.B of BC} - \text{Error at B}$$

$$= 107^\circ 20' - 1^\circ = 106^\circ 20'$$

$$\therefore \text{B.B of BC} = 106^\circ 20' + 180^\circ = \underline{286^\circ 20'}. \leftarrow \text{equal to the given data.}$$

\therefore corrected F.B and B.B are as follows:-

Line	F.B	B.B
AB	$75^\circ 0' 0''$	$255^\circ 0' 0''$
BC	$106^\circ 20' 0''$	$286^\circ 20' 0''$
CD	$224^\circ 50' 0''$	$44^\circ 50' 0''$
DA	$306^\circ 40' 0''$	$126^\circ 40' 0''$

Q.2c

Formation Level at P = 161.4 m. There is an uniformly

falling gradient of 1 in 50 from P to Q.

\therefore Formation level ^(F.L) at successive cross-sections are obtained

by deducting $\frac{1}{50} * 100 = 2\text{m}$ from level of preceding section.

$$\text{F.L at pie-zero m} = 161.4\text{m.}$$

$$\text{F.L at 100 metres} = 159.4\text{m}$$

$$\text{at 200 m} = 157.4\text{m}$$

$$\text{300 m} = 155.4\text{m}$$

$$\text{400 m} = 153.4\text{m.}$$

(at Q)

Thus, the depths of the embankments at various section

$$= \text{F.L} - \text{G.L}$$

$$\text{at P} = 161.4 - 153.0 = 8.4\text{m.}$$

$$\text{at 100m} = 159.4 - 151.8 = 7.6\text{m.}$$

$$\text{at 200m} = 157.4 - 151.2 = 6.2\text{m.}$$

$$\text{at 300m} = 155.4 - 150.6 = 4.8\text{m}$$

$$\text{at 400m} = 153.4 - 149.2 = 4.2\text{m}$$

ie. (at Q)

P.T.O -

Q. 2c continued

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C/S Area

$$\begin{aligned} \text{at P (zero metres)} (A_0) &= (b+sh) * h \\ &= [30 + (2 * 8.4)] * 8.4 \\ &= 393.12 \text{ m}^2 \end{aligned}$$

$$\begin{aligned} \text{at 100 metres} (A_{100}) &= [30 + (2 * 7.6)] * 7.6 \\ &= 343.52 \text{ m}^2 \end{aligned}$$

$$\begin{aligned} \text{at 200 metres} (A_{200}) &= [30 + (2 * 6.2)] * 6.2 \\ &= 262.88 \text{ m}^2 \end{aligned}$$

$$\begin{aligned} \text{at 300 metres} (A_{300}) &= [30 + (2 * 4.8)] * 4.8 \\ &= 190.08 \text{ m}^2 \end{aligned}$$

$$\begin{aligned} \text{at 400 metres} (A_{400}) &= [30 + (2 * 4.2)] * 4.2 \\ &= 161.28 \text{ m}^2 \end{aligned}$$

Applying Prismoidal Formula,

$$\text{Volume} = \frac{d}{3} \left(\begin{array}{l} \text{Area of 1st section} \\ + 4 \text{ times area of even sections} \\ + 2 \text{ times area of odd section} \\ + \text{Area of last section} \end{array} \right)$$

$$= \frac{100}{3} \left[393.12 + 4(343.52 + 190.08) + 2(262.88) + 161.28 \right]$$

$$\boxed{V = 107152 \text{ m}^3}$$

Q. 4c Correct length of chain at commencement = 20 m

Length of chain after chaining 1420 m = 20.04 m

$$\therefore \text{Mean length of chain while measuring} = \frac{20 + 20.04}{2} = 20.02 \text{ m}$$

True distance for wrong chainage of 1420 m

$$= \frac{20.02}{20} * 1420 = 1421.42 \text{ m}$$

Now, remaining distance = 2400 - 1420 = 980 m

Mean length of chain while measuring remaining distance = $\frac{20.04 + 20.08}{2}$

$$\text{True length of remaining 980 m} = \frac{20.06}{20} * 980 = 982.94 \text{ m}$$

Thus, total true distance = 1421.42 + 982.94 = 2404.36 m

Stn	B.S	I.S	F.S	Rise	Fall	RL	Remarks
1	2.150					450.00	BM
2	1.645		2.150 - 0.500 1.650	0.500		450.500	450 + 0.5 =
3		2.345		2.345 - 1.645 0.700		449.80	450.50 - 0.70
4	1.425		2.345 - 1.965 0.380			450.18	449.80 + 0.38
5	2.050		1.825	0.400		449.780	450.18 - 0.40
6		0.100		451.73 - 449.780 1.950		451.730	nc
7	(-) 1.690		0.10 - 0.12 (-) 0.020	0.120		451.850	(451.73 + 0.12) Inverted staff.
8	2.865		2.100		(-) 1.69 - 2.100 3.790	448.06	(451.85 - 3.79)
9			1.825	1.040		499.100	xxx
	$\Sigma BS = 8.445$		$\Sigma FS = \Sigma (FS 1 to 8)$			449.100	B

$$\text{Lost R.L} - 1^{\text{st}} \text{R.L} = \Sigma BS - \Sigma FS$$

$$449.10 - 450 = 8.445 - \Sigma FS$$

$$-0.9 = 8.445 - \Sigma FS$$

$$\Sigma FS = 9.345$$

* KINDLY CONSIDER the last R.L as 449.100.
 * By Mistake, in the Question Paper, it was printed as 499.100

Q.5a

Line length (metres)	WCB	R.B	N+ve, S-ve Latitude (L cos θ)	E+ve, W-ve. Departure (L sin θ)
AB 89.31	45° 10'	N45° 10' E	+ 62.967	+ 63.335
BC 219.76	72° 05'	N72° 05' E	+ 67.605	+ 209.103
CD 151.18	161° 52'	S18° 08' E	- 143.671	+ 47.051
DE 159.10	228° 43'	S48° 43' W	- 104.971	- 119.556
EA 232.26	300° 42'	N59° 18' W	+ 118.578	- 199.709

* If the student calculates corrected Latitudes & Departures, full marks should be given.
 * If student calculates only latitude & departure without corrections, the assessor may judiciously decide.

Q.5b

Interval = 15m

(5/6)

Dist	O(A)	15	30	45	60	75	90	105	120(B)	
offset		2.30	3.80	4.55	6.75	5.25	7.30	8.95	8.25	5.50

By Simpson's Rule,

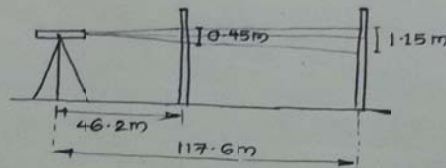
$$\text{Area} = \frac{d}{3} \left[\text{1st offset} + \text{Last offset} + 2 \left(\text{sum of remaining odd offsets} \right) + 4 \left(\text{sum of remaining even offsets} \right) \right]$$

$$= \frac{15}{3} \left[2.30 + 5.50 + 2(4.55 + 5.25 + 8.95) + 4(3.8 + 6.75 + 7.30 + 8.25) \right]$$

$$= \frac{15}{3} * (2.30 + 5.50 + 37.5 + 104.40)$$

$$A = 15/3 * 149.70 = 748.50 \text{ sq.m.} \therefore \boxed{A = 748.50 \text{ m}^2}$$

Q.6a



When telescope is horizontal & staff is held vertical

with reference to above sketch we have,

$$D = AS + B$$

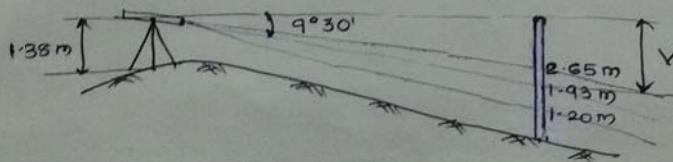
$$46.2 = A * 0.45 + B \quad \dots (i)$$

$$117.6 = A * 1.15 + B \quad \dots (ii)$$

Solving (i) & (ii) we get, $\boxed{A = 102, B = 0.3}$

A → Multiplying constant

B → Additive constant



When telescope is inclined downwards & staff is held vertical

Now, $S = 2.65 - 1.20 = 1.45 \text{ m}$ $\theta = 9.5^\circ$

$$PQ = A \cdot S \cdot \cos^2 \theta + B \cos \theta = (102 * 1.45 * \cos^2 9.5^\circ) + (0.3 * \cos 9.5^\circ)$$

$$\therefore \boxed{PQ = 144.17 \text{ m}}$$

$$\text{vertical component (V)} = \frac{AS \sin 2\theta}{2} + B \sin \theta$$

$$= \frac{102 * 1.45 * \sin(2 * 9.5^\circ)}{2} + 0.3 * \sin 9.5^\circ$$

$$= 24.08 + 0.05 = 24.13 \text{ m}$$

$$\therefore \text{R.L of Q} = \text{R.L of P} + \text{H.I} - \text{V} = 150 + 1.38 - 24.13 = 127.25 \text{ m}$$

Q.6b

(6/6)

Distance PQ = 180 m. R.L of BM = 100.125 m.

At P	At Q
$\theta_P = 64^\circ 30'$	$\theta_Q = 58^\circ 15'$
$\alpha_P = 20^\circ 43' 20''$	$\alpha_Q = 19^\circ 44' 55''$
B.S on BM = 1.625 m	B.S on BM = 1.125 m
H.I at P = 1.450 m	H.I at Q = 1.550 m

Soln:-

Line of collimation

$$\begin{aligned} \text{at P} &= \text{R.L of BM} + \text{B.S on BM} \\ &= 100.125 + 1.625 \\ &= 101.75 \text{ m} \end{aligned}$$

$$\begin{aligned} \text{R.L of P} &= 101.75 - \text{H.I at P} \\ &= 101.75 - 1.450 \end{aligned}$$

$$\boxed{\text{R.L of P} = 100.30 \text{ m}}$$

Line of collimation at Q

$$= \text{R.L of BM} + \text{B.S on BM}$$

$$\begin{aligned} &= 100.125 + 1.125 \\ &= 101.25 \text{ m} \end{aligned}$$

$$\begin{aligned} \therefore \text{R.L of Q} &= 101.25 - \text{H.I at Q} \\ &= 101.25 - 1.550 \\ &= 99.70 \text{ m} \end{aligned}$$

$$\therefore \boxed{\text{R.L of Q} = 99.70 \text{ m}}$$

$$\text{Now, } \theta_R = 180^\circ - \Sigma(\theta_P + \theta_Q)$$

$$\theta_R = 180^\circ - (64^\circ 30' + 58^\circ 15') \quad \boxed{\theta_R = 57^\circ 15'}$$

$$\text{dist PR} = \frac{\sin 58^\circ 15'}{\sin 57^\circ 15'} * \text{dist PQ} = 182 \text{ m}$$

$$\text{dist QR} = \frac{\sin 64^\circ 30'}{\sin 57^\circ 15'} * \text{dist PQ} = 193.17 \text{ m}$$

$$V_P = D_{PR} * \tan \alpha_P = 182 * \tan 20^\circ 43' 20'' = 68.85 \text{ m}$$

$$V_Q = D_{QR} * \tan \alpha_Q = 193.17 * \tan 19^\circ 44' 55'' = 69.34 \text{ m}$$

$$\left. \begin{aligned} \text{R.L of R} &= \text{R.L of P} + V_P = 100.30 + 68.85 = 169.15 \text{ m} \\ &= \text{R.L of Q} + V_Q = 99.70 + 69.34 = 169.04 \text{ m} \end{aligned} \right\}$$

$$\therefore \text{R.L of R} = \left(\frac{169.15 + 169.04}{2} \right) = 169.10 \text{ m}$$